

Memorandum

Subject: SD PFH 17-1(6) HILL CITY - LEAD

PAVEMENT RECOMMENDATIONS

Date: 12-08-2008

From:

Tracy D. Piparato, Highway Engineer /

Steve Deppmeier, Pavements Engineer

To:

Mark Meng, Project Manager Tony Galardi, Lead Designer

The project consists of four sites; Site 1, Site 2, Site 3, and Site 4. All four sites have an existing bituminous surface. The project will be 4R with a 20-year design life pavement structure, beginning at Station 31+69 and ending at Station 137+00. Although the roadway will be reconstructed at the spot repair sites, a pavement design life of 25 years (per PDDM 11.2.1.1) will not be used since portions of the roadway located between the spot repair sites will not be rebuilt. The combined distance for all four sites is approximately 1.3 miles. Site 1 begins at Station 31+69 and ends at Station 47+35. Throughout Site 1, the bituminous surfacing averaged 5.3 inches in thickness while the base course type material averaged 6.8 inches. The subgrade consisted predominantly of A-4 soils. The R-Value of the subgrade for this site was 29.

Site 2 begins at Station 100+00 and ends at Station 120+00. Throughout Site 2, the bituminous surfacing averaged 5.5 inches while the base course type material averaged 6.5 inches. The subgrade consisted of A-4, A-1-b, and A-2-4 soils. The R-Value of the subgrade for this site was 23.

Site 3 begins at Station 47+35 and ends at Station 64+81. Throughout Site 3, the bituminous surfacing averaged 7.8 inches while the base course type material averaged 3.5 inches. This section of roadway was more distressed with a large patch area caused from a failing dry-stack rock retaining wall adjacent to the roadway. The subgrade consisted mainly of A-4 and A-2-4 soils. The R-Value of the subgrade for this site was 19. The existing dry-stack rock retaining wall will be reconstructed using an MSE wall resulting in the reconstruction of the road embankment as well.

Site 4 begins at Station 120+00 and ends at Station 137+00. Throughout Site 4, the bituminous surfacing averaged 7.6 inches while the base course type material averaged 4.8 inches. This section of roadway has a large patch area caused from a failing dry-stack rock retaining wall adjacent to the roadway. The subgrade consisted predominantly of A-6 soils. Similar to Site 3,





the existing dry-stack rock retaining wall will be reconstructed with an MSE wall resulting in the reconstruction of the road embankment as well.

Two-way ADT, supplied by Pennington County Highway Department, was approximately 2380 with 5% of vehicles larger than a passenger vehicle. Due to the large number of logging trucks and heavy construction/mining trucks utilizing this route, a value of 1,246,700 ESALs was used to calculate the structural number (SN). The design subgrade soil resilient modulus was estimated to be an average of 5700 psi for a design R-Value of 24 across the entire project. The required SN is 3.64. The following pavement structural sections were evaluated:

Option 1: Recommended
5 inches HACP
12 inches Base Course
SN = 3.68

Cost Estimate = \$1,093,798 per mile for 28 foot width, paving items only (See attached DARWin Pavement Calculations)

Option 2:

6 inches HACP 10 inches Base Course SN = 3.80 Cost Estimate = \$1,116,487 per mile for 28 foot width, pay

Cost Estimate = \$1,116,487 per mile for 28 foot width, paving items only (See attached DARWin Pavement Calculations)

The above two options are 20-year designs. The cost estimates are relative costs pertaining to HACP and base course items only.

Since the 4 sites may not be included in one contract, the following bid items are recommended:

PAVEMENT MATERIALS

- HACP should be Bid Item 40301-0000, Hot Asphalt Concrete Pavement. The unit weight can be estimated at 145.2 lb/ft³.
- The asphalt cement should be PG 58-34. Quantity can be estimated at 6.0% by weight of mix.
- The 5 inch depth HACP shall be placed in two equal lifts.
- Tack Coat is required between lifts. The emulsion type should be CSS-1, CSS-1h, SS-1, or SS-1h. The application rate for estimating is 0.10 gal/yd².
- A Fog Seal, Bid Item 409 should be applied to the final surface. The emulsion type should be CSS-1, CSS-1h, SS-1, or SS-1h. The application rate for estimating is 0.10 gal/yd².
- Base Course should be Bid Item 30101-0000, Aggregate Base, paid by the ton. The unit weight can be estimated at 139 lb/ft³.
- Bid Item 41101-0000, Prime Coat paid by the ton, should be applied to the base course prior to paving. The application rate for estimating is 0.33 gal/yd².
- Blotter, Item 411 should be included in the contract. For estimating, the application rate is 14.75 lb/yd².

CC:

Mike Voth, Pavements FDL

Dave Balding, COE

Mike Peabody, Materials Engineer Richard Duval, QA Engineer

Matt DeMarco, Lead Geotechnical Engineer

Attachment:

DARWin Pavement Calculations

Cost Estimate Worksheet

LTPPBind

1993 AASHTO Pavement Design

DARWin Pavement Design and Analysis System

A Proprietary AASHTOWare Computer Software Product

Flexible Structural Design Module

SD PFH 17-1(6) HILL CITY - LEAD OVERALL PROJECT OPTION 1

Flexible Structural Design

18-kip ESALs Over Initial Performance Period	1,246,070
Initial Serviceability	4.2
Terminal Serviceability	2.5
Reliability Level	75 %
Overall Standard Deviation	0.49
Roadbed Soil Resilient Modulus	5,700 psi
Stage Construction	1
Calculated Design Structural Number	3.64 in

Rigorous ESAL Calculation

Performance Period (years)	20
Two-Way Traffic (ADT)	2,380
Number of Lanes in Design Direction	1
Percent of All Trucks in Design Lane	100 %
Percent Trucks in Design Direction	60 %

			Average Initial	Annual %	Accumulated
	Percent	Annual	Truck Factor	Growth in	18-kip ESALs
Vehicle	of	%	(ESALs/	Truck	over Performance
Class	<u>ADT</u>	<u>Growth</u>	Truck)	<u>Factor</u>	<u>Period</u>
2	95	2	0.0004	0	4,717
6	1	2	1.2	0	148,962
9	3	2	2.2	0	819,293
10	1	2	2.2	0.	273,098
Total	100	-	-	-	1,246,070

Growth Simple

Total Calculated Cumulative ESALs 1,246,070

Specified Layer Design

		Struct	Drain			
		Coef.	Coef.	Thickness	Width	Calculated
<u>Layer</u>	Material Description	(<u>Ai</u>)	(Mi)	(Di)(in)	<u>(ft)</u>	<u>SN (in)</u>
1	НАСР	0.4	1	5	-	2.00
2	BASE COURSE	0.14	1	12	-	1.68
Total	-	-	-	17.00	-	3.68

1993 AASHTO Pavement Design

DARWin Pavement Design and Analysis System

A Proprietary AASHTOWare Computer Software Product

Flexible Structural Design Module

SD PFH 17-1(6) HILL CITY - LEAD OVERALL PROJECT OPTION 2

Flexible Structural Design

18-kip ESALs Over Initial Performance Period	1,246,070
Initial Serviceability	4.2
Terminal Serviceability	2.5
Reliability Level	75 %
Overall Standard Deviation	0.49
Roadbed Soil Resilient Modulus	5,700 psi
Stage Construction	1
Calculated Design Structural Number	3.64 in

Rigorous ESAL Calculation

Performance Period (years)	20
Two-Way Traffic (ADT)	2,380
Number of Lanes in Design Direction	1
Percent of All Trucks in Design Lane	100 %
Percent Trucks in Design Direction	60 %

			Average Initial	Annual %	Accumulated
	Percent	Annual	Truck Factor	Growth in	18-kip ESALs
Vehicle	of	%	(ESALs/	Truck	over Performance
<u>Class</u>	<u>ADT</u>	<u>Growth</u>	Truck)	<u>Factor</u>	<u>Period</u>
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Total	100	-	•	-	1,246,070

Growth Simple

Total Calculated Cumulative ESALs 1,246,070

Specified Layer Design

		Struct Coef.	Drain Coef.	Thickness	337: 341-	01.1.1
_					Width	Calculated
<u>Layer</u>	Material Description	<u>(Ai)</u>	(Mi)	(Di)(in)	<u>(ft)</u>	<u>SN (in)</u>
1	HACP	0.4	1	6	-	2.40
2	BASE COURSE	0.14	1	10	_	1.40
Total	-	-	-	16.00	-	3.80

SD PFH 17-1(6) - HILL CITY - LEAD

Paved width = 28 ft. Route = 1.33 miles

	\$574,751	\$689,701	\$713,735	\$11,896	\$856,482	\$14,275		\$5,441	\$5,441	\$22,091	\$6,447	\$39,419
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\$ / ton	40	40	120	200	120	200	\$ / ton	009	009	775	40	UBTOTAL
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tons	14369	17243	5948	59	7137	71	tons	တ	တ	73	161	
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lbs to tons	2000 =	2000	2000		2000		density	241	241	253	2000	
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unit weight	139	139	145.2		145.2		lication r	0.1	0.1	x 0.33 /	14.75	
= !	×	×	×		×		app	×	×	×	×	
width (ft)	35.33	35.33	28		28			21,853	21,853	21,853	21,853	
	ì	·×	×		×			n	II	ti	H	
length of route (ft	7025	7025	7025		7025		square yards	0.1111	0.1111	0.1111	0.1111	
len	×	×	×		×		vert to	×	×	×	×	
to feet	0.0833	0.0833	0.0833		0.0833		re feet con	28	28	x 28 x 0.1111	28	
inches convert to feet	×	×	×		×		sdna	×	×	×	×	
inches	10	12	5	1%	9	1%		7025	7025	7025	7025	
Option Items	10" Base Imported	12" Base Imported	5" HACP	Lime	6" HACP	Lime		Tack	Fog	Prime	Blotter	

Option #1 5" HACP + 12" Base Imported

\$1,454,751 total cost

\$1,093,798 per mile

\$1,484,927 total cost

6" HACP + 10" Base Imported

Option #2

\$1,116,487 per mile

BindReport-SD17-1(6).txt

Five Closest Weather Stations For Latitude/Longitude= 44.006/103.324 Report Date: 12/4/2008 (LTPPBind v3.0 Alpha)

SD 17-1(6) - HILL CITY - LEAD

E=34 km SD2231 pennington deerfield 3 se 1716 44 ,103.78	Mean (Std, N) 28.5 (19,16) -36.1 (42,16) 30.2 (36,16) 1785 (191,16)	High Low Rel. 45.1 –26.5 46-28 (74,65) 46-34 (74,97) 52-34 (98,97) 52-40 (98,98)
<pre>D=20 km SD6937 Dennington Tapid city 396 44.05 ,103.07 L997</pre>	Mean (Std, N) 34.8 (21,35) -28.6 (31,35) 30.7 (42,35) 2653 (221,35)	High Low Rel. 53.6 -21.2 58-22 (98,61) 58-28 (98,98)
C=18 km SD5870 pennington mt rushmore nat 1 1466 43.88 ,103.45	Mean (Std, N) 31.8 (23,35) -27.3 (39,35) 28.5 (42,35) 2161 (248,35)	l. High Low Rel. 49.0 -20.2 52-22 (98,70) 52-28 (98,98)
B=14 km SD6947 pennington rapid city 977 44.12 ,103.28	Mean (Std, N 34.2 (21,33) -28 (32,33) 31.4 (39,33) 2633 (260,33)	High Low Rel. 53.4 -20.8 58-22 (98,66) 58-28 (98,98)
A=14 km SD6427 pennington pactola dam 1337 44.07 ,103.48	Mean (Std, N) 30.6 (21,32) -30.9 (32,31) 32.6 (45,31) 2006 (215,32)	High Low Rel. 47.4 -22.8 52-28 (98,95) 52-34 (98,98)
General Station ID County/District Weather Station Elevation, m Latitude, Longitude Last Year Data Available	Air Temperature High Temperature Low Temperature Low Temperature Drop Degree-Days > 10C	PG Pavement Temperature, C 50% Reliability PG >50% Reliability PG = = =